GENERAL NOTES

- DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH PROVISIONS OF THE WISCONSIN ENROLLED COMMERCIAL BUILDING CODE (WECBC/IBC2009).
- 2. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE COMPLETED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR BRACING AND SHORING (WITHOUT OVERSTRESSING) ALL STRUCTURAL ELEMENTS AS NECESSARY AT ANY STAGE OF CONSTRUCTION UNTIL COMPLETION OF THE PROJECT. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, SEQUENCES OR PROCEDURES OF CONSTRUCTION. CONTRACTOR SHALL RECOGNIZE AND CONSIDER EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING CONSTRUCTION PERIOD
- 3. CONTRACTOR SHALL SUBMIT THE FOLLOWING COMPONENT DESIGNS TO THE STATE OF WISCONSIN AS DEFERRED SUBMITTALS TO FOLLOW PERMIT DOCUMENTS: STEEL DECK AND FASTENING, AND STEEL BEAM/COLUMN MOMENT CONNECTIONS AS INDICATED ON FRAMING DRAWINGS, PRECAST HOLLOW CORE PLANK & COMPONENTS, PERFORMANCE SPECIFIED STEEL STAIRS, AND COLD-FORMED STEEL STUDS.
- 4. CONSULT ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS & TRADES FOR VERIFICATION OF LOCATION AND DIMENSIONS OF CURBS, DEPRESSIONS, DOOR CLOSERS, AND OTHER PROJECT REQUIREMENTS NOT SHOWN ON STRUCTURAL DRAWINGS.
- 5. GC SHALL VERIFY AND COORDINATE DIMENSIONS AND DETAILS AS SHOWN ON THE DRAWINGS. WHEN DISCREPANCIES OR QUESTIONS ARISE, THE A/E SHALL BE IMMEDIATELY NOTIFIED FOR RESOLUTION.
- 6. VERIFY SIZE AND LOCATION OF ALL ROOF, FLOOR, AND WALL OPENINGS WITH MECHANICAL AND ELECTRICAL CONTRACTORS. OPENINGS LESS THAN 12 INCHES IN DIMENSION ARE GENERALLY NOT SHOWN.
- 7. SEE A001 AND A002 FOR PROJECT PHASING INFORMATION.
- 8. REFERENCE PROJECT SPECIFICATIONS FOR MATERIALS AND OTHER REQUIREMENTS NOT NOTED ON THE DRAWINGS.
- 9. CONTRACTOR TO PROVIDE TEMPORARY SHORING AS REQUIRED DURING THE DEMOLITION/NEW CONSTRUCTION PROCESS. SHORING DESIGN SHALL BE BY GC, AS REQUIRED TO ACCOMMODATE PROJECT PHASING AND G.C. MEANS AND METHODS.

DESIGN DATA

ROOF LIVE LOAD:

1.	1.1. SNOW LOAD — BUILDING —
2.	WIND LOAD: 2.1. METHOD: SIMPLIFIED PROVISION FOR LOW—RISE BUILDING 2.2. BUILDING ENCLOSURE: FULLY ENCLOSED 2.3. CANOPY ENCLOSURE: OPEN 2.4. MAIN WIND FORCE RESISTING SYSTEM: 2.4.1. BASIC WIND SPEED
3.	SEISMIC: ANALYSIS PROCEDURE: "EQUIVALENT LATERAL FORCE PROCEDURE" 3.1. SEISMIC OCCUPANCY CATEGORY
4.	FLOOR LIVE LOADS: 4.1. OFFICE FLOORS 50 PSF 4.2. CORRIDORS 100 PSF
5.	MATERIAL STRENGTHS: 5.1. CONCRETE (F'c @ 28-DAYS) 5.1.1. FOOTINGS
	5.2. REINFORCING STEEL (Fy): 5.2.1. REBAR — ASTM A615/A775, GR 60 5.2.2. WELDED WIRE FABRIC — ASTM A185, PLAIN
	5.3. STRUCTURAL STEEL (Fy): 5.3.1. ALL STEEL SHAPES — ASTM A992, GR. 50 5.3.2. HOLLOW STRUCTURAL STEEL SHAPES — ASTM A500B 5.3.3. STEEL JOISTS — ASTM A572, GR. 50 5.3.4. METAL DECK — ASTM A611, GRC, D, OR E 5.3.5. HIGH STRENGTH BOLTS — ASTM F1554 Gr. 36

GEOTECHNICAL

Revisions:

VA FORM 08-6231, OCT 1978

1. FOR DETAILED DESCRIPTION OF SOILS, FOUNDATION SUBGRADE PREPARATION, AND HELICAL PIER

5.3.8. WELDING ELECTRODES —

5.3.6. ANCHOR BOLTS — ASTM F1554 Gr. 36

5.3.7. HEADED ANCHOR RODS — ASTM A307, GR.-A

6. SOIL BEARING PRESSURE — 3,000 PSF

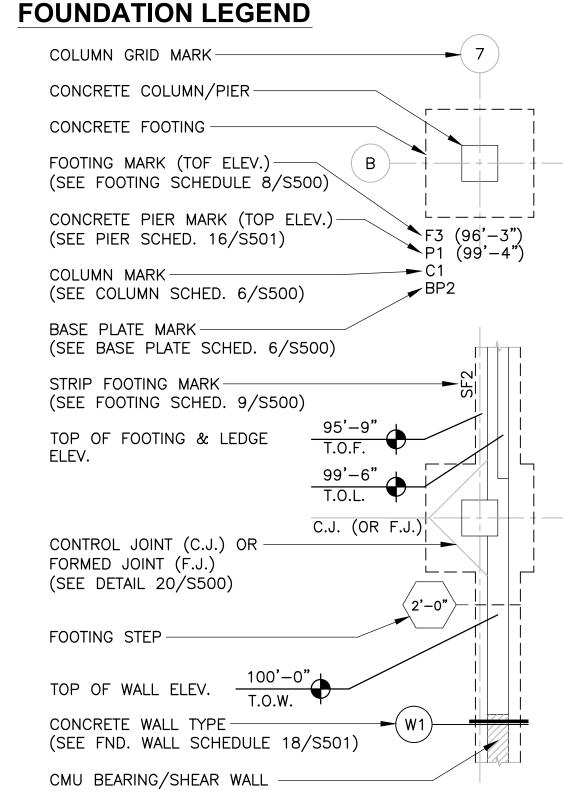
(PER GILES ENGINEERING ASSOCIATES GEOTECHNICAL REPORT)

RECOMMENDATIONS REFER TO:

1.1. GEOTECHNICAL REPORT & RECOMMENDATIONS DATE: JANUARY 24, 2014 REPORT/JOB NO.: 1G-1312013 BY GEOTECHNICAL CONSULTANT GILES ENGINEERING ASSOCIATES, INC. JEFFREY S. MILLER, PE N8 W22350 JOHNSON DRIVE SUITE A1 WAUKESHA, WI 53186 (262) 544-0118

EARTHWORK & FOUNDATIONS

- 1. CENTER COLUMN FOOTINGS ON COLUMN CENTERLINES UNLESS OTHERWISE NOTED.
- 2. WALL FOOTINGS ARE CENTERED ON FOUNDATION WALL UNLESS OTHERWISE NOTED.
- WALL FOOTINGS ARE 12 INCHES THICK AND 8 INCHES WIDER THAN THE WALL ABOVE (FOOTINGS PROJECT 4 INCHES BEYOND WALL FACE) UNLESS OTHERWISE NOTED.
- 4. ELEVATIONS NOTED ON PLANS ARE TO THE TOP OF FOOTING (TOF).
- 5. BOTTOM OF FOOTINGS SHALL BE 4'-0" MINIMUM BELOW EXTERIOR FINISH GRADE, UNLESS OTHERWISE NOTED.
- COLUMN AND WALL FOOTINGS SHALL BEAR ON NATIVE, UNDISTURBED SOIL, SOIL IMPROVED AREAS, OR COMPACTED FILL AS DEFINED IN THE PROJECT SPECIFIC GEOTECHNICAL REPORT, AT ELEVATIONS INDICATED ON THE DRAWINGS.
- SLAB-ON-GRADE SHALL BE UNDERLAIN BY A VAPOR BARRIER IF SPECIFIED, AND SIX INCHES MINIMUM OF COARSE GRANULAR (COMPACTED) FILL MATERIAL. REFERENCE GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION.
- 8. MAINTAIN GRAVEL THICKNESS, SLAB DEPTH, REINFORCEMENT, AND REINFORCEMENT POSITION AT DROPPED OR THICKENED SLAB ON GRADE.
- REINFORCE ALL SLABS ON GRADE WITH WELDED WIRE FABRIC AS DEFINED ON THE PLANS, POSITIONED OR SUPPORTED TO BE IN THE TOP THIRD OF THE SLAB UNLESS NOTED OTHERWISE.
- 10. BACKFILL AROUND THE EXTERIOR FOUNDATION WALLS WITH (A FREE DRAINING GRANULAR MATERIAL TO THE ELEVATION OF THE ROUGH GRADE).
- 11. CONTRACTOR TO KEEP EXCAVATIONS DRY AND PROTECTED FROM FROST AT ALL TIMES DURING THE FOUNDATION CONSTRUCTION.
- 12. NOTIFY A/E IF NATURE OF SOIL AT DEPTHS SHOWN DOES NOT APPEAR SUITABLE FOR FOUNDATIONS.



CAST-IN-PLACE CONCRETE

- 1. CONCRETE REINFORCING SHALL HAVE THE FOLLOWING MINIMUM PROTECTIVE COVER:
- 1.1. CONCRETE IN CONTACT OR PLACED ON EARTH ——— 3" 1.2. CONCRETE EXPOSED TO EARTH OR WEATHER -- 1 1/2" CONCRETE WITH INTERIOR EXPOSURE — 3/4" 1.4. CONCRETE PIERS PRIMARY REINFORCEMENT, TIES, AND
- CONDUITS, PIPES, DUCTS, OR FIXTURES SHALL NOT BE PLACED IN CONCRETE COLUMNS, PIERS OR BEAMS (UNLESS SPECIFIC REVIEW AND APPROVAL IS MADE BY THE A/E).
- 3. APPROVED SLEEVES, CONDUITS, OR PIPES THROUGH SLABS

- AND WALLS SHALL BE PLACED SO THAT THEY ARE NO CLOSER THAN THREE DIAMETERS ON CENTER AND THEY DO NOT DISPLACE REINFORCING.
- 4. DO NOT CUT OR PLACE HOLES IN CONCRETE SLABS, BEAMS, COLUMNS OR WALLS WITHOUT PRIOR APPROVAL OF THE A/E.
- 5. BARS SHALL BE SPLICED PER DETAILS WHERE PROVIDED. OTHERWISE BARS SHALL BE CLASS "B" LAP SPLICED IN LONGEST CONVENIENT LENGTHS WITH ADJACENT LAPS, AS PER TABLE THIS SHEET, STAGGERED 3'-0" MINIMUM. BARS SHALL BE CONTACT SPLICED OR SPACED A MINIMUM DISTANCE APART OF THE LESSER OF 1/5TH THE LAP LENGTH OR SIX INCHES.
- 6. CLEAR SPACING BETWEEN REINFORCING (UNLESS SHOWN TO BE CONTACT LAP SPLICED) SHALL BE A MINIMUM OF 1 ½ BAR DIAMETERS, 1 1/2", OR 1 1/3 TIMES THE DIAMETER OF THE AGGREGATE, WHICHEVER IS GREATER.
- 7. EACH LAYER OF STEEL SHALL BE PLACED AS CLOSE TO EACH FACE AS POSSIBLE, MEETING MINIMUM COVER REQUIRED ABOVE, UNLESS NOTED OTHERWISE.
- 8. EMBEDMENT LENGTHS, COMPRESSION SPLICE LENGTHS AND LAP LENGTHS FOR TENSION SPLICES CLASS "A" AND "B" SHALL CONFORM TO THOSE OF CRSI "REINFORCEMENT ANCHORAGE AND SPLICES", CURRENT EDITION.
- 9. PROVIDE COLUMN OR WALL DOWELS OF THE SAME SIZE AND NUMBER AS THE RESPECTIVE COLUMNS OR WALL
- 10. ALL REINFORCING HOOKS NOT NOTED SHALL BE ACI STANDARD HOOKS.

REINFORCING, UNLESS NOTED OTHERWISE.

- 11. WHERE DOWELS MUST BE PLACED FLUSH WITH CONCRETE AND THEN BENT OUT FOR SUBSEQUENT WORK, GRADE 40 REINFORCEMENT STEEL OR COUPLERS SHALL BE USED AND SPACING REDUCED TO ACHIEVE EQUIVALENT STRENGTH.
- 12. NO TACK WELDING WILL BE PERMITTED ON GRADE 40 OR 60 REINFORCING STEEL.
- 13. CONCRETE SLAB REINFORCING SHALL BE REINFORCED WITH 6X6 - W2.9XW2.9 WWF, UNLESS NOTED OTHERWISE.
- 14. CONTRACTOR SHALL NOTIFY THE ARCHITECT AT LEAST 24 HOURS PRIOR TO PLACING CONCRETE, FOR EACH POUR.
- 15. ALL CONSTRUCTION JOINTS SHOWN SHALL BE INCORPORATED INTO STRUCTURE UNLESS THEIR ELIMINATION IS APPROVED BY THE A/E, ADDITIONAL CONSTRUCTION JOINTS REQUIRED TO FACILITATE CONSTRUCTION SHALL BE LOCATED AND DETAILED ON SHOP DRAWINGS AND ARE SUBJECT TO A/E APPROVAL
- 16. PROVIDE 2-#5 BARS AROUND ALL OPENINGS AND 2-#5 DIAGONALLY AT ALL OPENING CORNERS UNLESS OTHERWISE SPECIFIED. EXTEND 2'-6" PAST OPENING, TYPICAL.
- 17. CONCRETE PIERS/PILASTERS SHALL BE POURED MONOLITHICALLY WITH CONCRETE WALLS.
- 18. ALLOW AT LEAST 24 HOURS BETWEEN POURING ADJACENT WALL SECTIONS BETWEEN CONSTRUCTION JOINTS.
- 19. HORIZONTAL WALL REINFORCEMENT SHALL BE MADE CONTINUOUS AT ALL CORNERS OR CORNER BARS PROVIDED.
- 20. ANCHOR BOLTS SHALL BE SET AND CONCRETE BEARING SURFACE FOR COLUMNS SHALL BE FINISHED TO THE
- 20.1. ELEVATION OF CONCRETE SURFACE PLUS OR MINUS 3/8"
- 20.1. ELEVATION TOP OF ANCHOR BOLTS PLUS 1" OR MINUS
- 20.1. OUT OF POSITION OF ANCHOR BOLTS PLUS OR MINUS 20.2. ADDITIONAL WORK REQUIRED BY A/E FOR EVALUATION & SUBSEQUENT DESIGN REVISIONS ARISING OUT OF IMPROPERLY INSTALLED ANCHOR BOLTS SHALL BE AT

THE GC'S EXPENSE, AND BASED ON THE APPLICABLE

- CONSULTANT LABOR CODE & ASSOCIATED RATE SCHEDULE. 21. REFER TO OTHER TRADE'S DRAWINGS FOR LOCATION AND DIMENSIONS OF CONCRETE REVEALS, NOTCHES, REGLETS, DRIPS, PADS, CURBS, CHAMFER BLOCKOUTS AT DOORWAYS
- AND ALL OTHER PROJECT REQUIREMENTS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- 22. ALL CONCRETE SUBJECT TO FREEZE-THAW SHALL BE AIR-ENTRAINED TO $6\% \pm 1\%$.

PRECAST CONCRETE

FOLLOWING TOLERANCE(S):

- . COMPLY WITH MNL-116 AND /OR MNL-117 OF THE PRECAST CONCRETE INSTITUTE, ACI-318, AND "RECOMMENDATIONS FOR CONCRETE MEMBERS PRESTRESSED WITH UNBONDED TENDONS" BY ACI-ASCE JOINT COMMITTEE
- PRECAST, PRESTRESSED MEMBERS SHALL BE DESIGNED FOR ERECTION LOADS, INCLUDING SUPERIMPOSED LOADS SHOWN ON THE DRAWINGS.
- 3. PRECAST MANUFACTURER IS TO DESIGN, PROVIDE CALCULATIONS, AND FURNISH STEEL HEADERS FOR OPENINGS AS NECESSARY OR AS SHOWN ON THE DRAWINGS. REFER TO THE ARCHITECTURAL DRAWINGS FOR FIRE PROTECTION REQUIREMENTS FOR THE STEEL HEADERS.
- WELDERS FOR STEEL CONNECTIONS IN PRECAST SHALL BE CERTIFIED IN ACCORDANCE WITH AWS D1.1.
- 5. REFER TO ARCHITECTURAL DRAWINGS FOR FIRE RATING REQUIREMENTS. ALL MEMBERS, PLANKS AND BEAMS, SHALL BE DESIGNED FOR UNRESTRAINED CONDITIONS.
- 6. THE PRECAST MANUFACTURER SHALL SUBMIT FOR REVIEW CERTIFIED SHOP DRAWINGS AND DESIGN CALCULATIONS PREPARED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER WHO IS REGISTERED IN THE STATE OF WISCONSIN.

7. PRECAST ELEMENTS USED AS A DIAPHRAGM SYSTEM SHALL BE DESIGNED TO RESIST ALL LATERAL FORCES AS NOTED ON THE DOCUMENTS. THE PRECAST MANUFACTURER SHALL BE RESPONSIBLE FOR THE COMPLETE DESIGN OF THE PRECAST COMPONENTS AND ALL CONNECTIONS NECESSARY TO PROVIDE A FULLY FUNCTIONAL DIAPHRAGM.

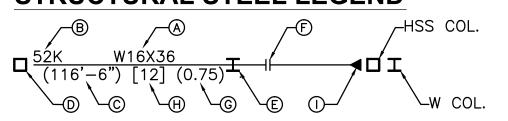
STRUCTURAL STEEL

- 1. PROVIDE MINIMUM OF L3X3X3/8 CLIP ANGLES WELDED TO COLUMNS FOR SUPPORT OF METAL ROOF OR FLOOR DECK/PLANK WHERE REQUIRED.
- 2. PROVIDE A WELDED ANGLE FRAME AT ALL ROOF DUCT AND ROOF DRAIN OPENINGS, AS APPLICABLE.
- 3. USE CONNECTIONS AS DETAILED ON PLANS OR THE STANDARD GUIDE DETAILS PROVIDED WITH THE CONTRACT DOCUMENTS. WHENEVER CONNECTIONS ARE NOT COVERED, THE FABRICATOR SHALL REQUEST THE A/E SUPPLY A CONNECTION DETAIL.
- 4. COLUMN CAP PLATES ARE 1/2" THICK UNLESS NOTED OTHERWISE. SLOPE TO MATCH BEAM SLOPE.
- 5. PROVIDE 3/8" STIFFENER PLATES ON EACH SIDE OF BEAMS OVER COLUMNS, UNLESS SPECIFICALLY NOTED TO OMIT.
- 6. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL MISCELLANEOUS STEEL
- 7. ALL STRUCTURAL STEEL IS TO BE FIREPROOFED AS INDICATED ON ARCHITECTURAL DRAWINGS.

HELICAL STEEL PIERS

- 1. REFERENCE GEOTECHNICAL REPORT & RECOMMENDATIONS AND ADDENDUM DATED MARCH 7, 2014 FOR ANTICIPATED HELICAL PIER INSTALLATION VALUES.
- 2. HELICAL PIERS SHALL BE INSTALLED AT LOCATIONS INDICATED TO AN ALLOWABLE CAPACITY OF 25 KIPS.
- 3. ESTIMATED HELICAL PIER TIP ELEVATION = 600'-0", BASED UPON (3) HELIX PLATES OF DIAMETER 8", 10", AND 12" AND FACTOR OF SAFETY = 2.0.
- 4. HELICAL PIER SHAFTS TO BE SIZED AS NECESSARY FOR INSTALLATION TORQUES REQUIRED TO MEET SPECIFIED SERVICE LOAD CAPACITY AT ANTICIPATED DEPTH.
- 5. REFERENCE SPECIFICATION SECTION 316615 AN 316615A (HELICAL FOUNDATION PILES) FOR SPECIFIC REQUIREMENTS INCLUDING BUT NOT LIMITED TO SUBMITATALS, PRODUCTS, INSTALLATION PROCEDURES, AND ALLOWABLE TOLERANCES.

STRUCTURAL STEEL LEGEND



A) INDICATES BEAM SIZE B) INDICATES CONNECTION DESIGNED BY FABRICATOR TO TRANSFER 52K VERTICAL LOAD AT LOCATION SPECIFIED C)INDICATES TOP OF STEEL BEAM (OR JOIST) = 116'-6''INDICATES BEAM FRAMING INTO FACE OF COLUMN DINDICATES BEAM CONTINUOUS OVER COLUMN

DINDICATES MOMENT CONNECTION

) INDICATES TYPICAL SHEAR SPLICE INDICATES REQUIRED UPWARD CAMBER IN BEAM (INCHES)) INDICATES NUMBER OF 3/4" X 5" WHS PER SPAN

ABOVE FINISH FLOOR MAXIMUM ALTERNATE MECHANICAL ANCHOR BOLT MINIMUM ARCHITECT (URAL) ARCH MISC MISCELLANEOUS BEARING MOMENT CONNECTION BELOW FINISH FLOOR NF NEAR FACE BOTTOM NOMINAL NOT APPLICABLE BOTTOM CHORD BOTTOM OF CONCRETE NOT IN CONTRACT BOTTOM OF FOOTING NOT TO SCALE BOTTOM OF STEEL ON CENTER OPENING BUILDING OPPOSITE CAST-IN-PLACE OUTSIDE DIMENSION OUTSIDE FACE COLUMN OUT-TO-OU CONCRETE **OVERHEAD** CONCRETE MASONRY UNI CONNECTION PERIMETER CONST CONSTRUCTION **PLUMBING** CONTINUOUS CONTR CONTRACTOR POINT CONTROL JOINT P/T POST-TENSIONED COORD COORDINATE POUNDS DEAD LOAD POUNDS PER CUBIC FOOT PCF | DEMO PCI POUNDS PER CUBIC INCH DEMOLITION PLF | POUNDS PER LINEAR FOOT DETAIL PSF POUNDS PER SQUARE FOOT DIAMETER PSI POUNDS PER SQUARE INCH DIMENSION PRECAST (OR) PRESTRESSED DOWEL BAR ANCHORS PREFABRICATED DRAWING PROJECTION EACH FACE **RADIUS** EACH WAY REFERENCE EDGE OF DECK REINFORCING REQUIRED EDGE OF SLAB **ELECTRICAL** RETAINING WALL REVISED (OR)REVISION ELEVATION **ENGINEER** ROOF DRAIN ROOF TOP UNIT **EXIST EXISTING** SCHEDULE **EXPANSION** SLIP-CRITICAL **EXPANSION JOINT** SHEET **EXTERIOR** SLBB | SHORT LEGS BACK-TO-BACK EFOW | EXTERIOR FACE OF WALL SHORT WAY FACE OF SIMILAR FAR FACE SLAB-ON-GRADE FINISH(ED FIN SNOW LOAD FLOOR SPACE SPA FLOOR DRAIN SPECIFICATION FOOTING SQUARE FDN FOUNDATION STAINLESS STEEL STANDARD GAUGE GALVANIZED STEEL STRUCTURE(AL) GENERAL CONTRACTOR HEADED WELDED STUD **SYMMETRICAL TEMPORARY** HEATING, VENTILATING, AND AIR CONDITIONING TENSION—CONTROL THICK(ENDED)(NESS) HOLLOW STRUCTURAL SHAPE TOP HOR TOP CHORD HORIZONTAL INFO INFORMATION TOP OF CONCRETE TOP OF FOOTING INSIDE DIAMETER TOP OF LEDGE INSIDE FACE INSUL TOP OF PIER INSULATION TOP OF STEEL INTERIOR TOP OF WALL JOIST BEARING ELEVATION TOW KIP (1,000 POUNDS) **TYPICAL** KIPS PER SQUARE INCH UNLESS NOTED OTHERWISE UNO LENGTH (OR) SPAN VAPOR RETARDER VERIFY IN FIELD LIVE LOAD LOCATION VERTICAL WEIGHT LONG LEG VERTICAL LLBB | LONG LEGS BACK-TO-BACK | WWF WELDED WIRE FABRIC WIDTH/WIDE LONG LEG HORIZONTAL LONG WAY WIND LOAD MANUFACTURER WITH **WITHOUT** MASONRY OPENING MASONRY SHEAR WALL WORK POINT

STANDARD ABBREVIATIONS

ARCHITECT/ENGINEER MATL

BAR SIZE	COLUMNS		BEAMS						SLABS, OTHER					
			f'c = 4 KSI			f'c = 5 KSI			f'c = 4 KSI			f'c = 5 KSI		
	f'c = 4 KSI	f'c = 5 KSI	BOT. BARS	TOP BARS	TOP* BARS									
#3	16	16	16	18	24	16	16	21	16	18	24	16	16	21
#4	19	17	19	24	32	17	22	28	19	24	32	17	22	28
#5	23	21	23	30	40	21	27	35	23	30	40	21	27	35
#6	28	25	28	36	47	25	33	42	28	36	47	25	33	42
#7	33	29	33	42	55	29	38	50	33	43	57	30	39	51
#8	39	35	39	51	67	35	46	60	44	57	75	39	51	67
#9	50	44	50	64	84	44	58	75	56	72	94	50	65	84
#10	88	79	63	82	107	56	73	96	70	92	120	63	82	107
#11	108	97	77	100	131	69	90	117	86	112	147	77	101	131

June 23, 2014

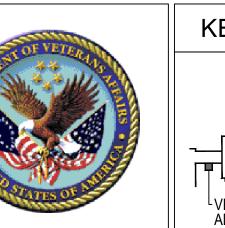
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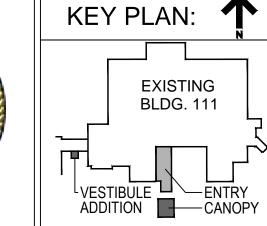
KJM



Dept. of Veterans Affairs **Medical Center** 5000 W. National Avenue Milwaukee, WI











PROJECT LEADER/ARCHITECT nequamegon ASHLAND, WI • MILWAUKEE, WI • MADISON, WI • DULUTH, MN (715) 682-6004 (414) 258-6004 (608) 333-8940 (218) 728-4293 WWW.CHEQBAYGRP.COM

NOTES Approved: Project Director

Project Title **Drawing Title** Building 111 -STRUCTURAL GENERAL Renovate South Entry Milwaukee, Wisconsin

Project Number 695-12-101SCP **Building Number** Drawing Number Checked By: Drawn By:

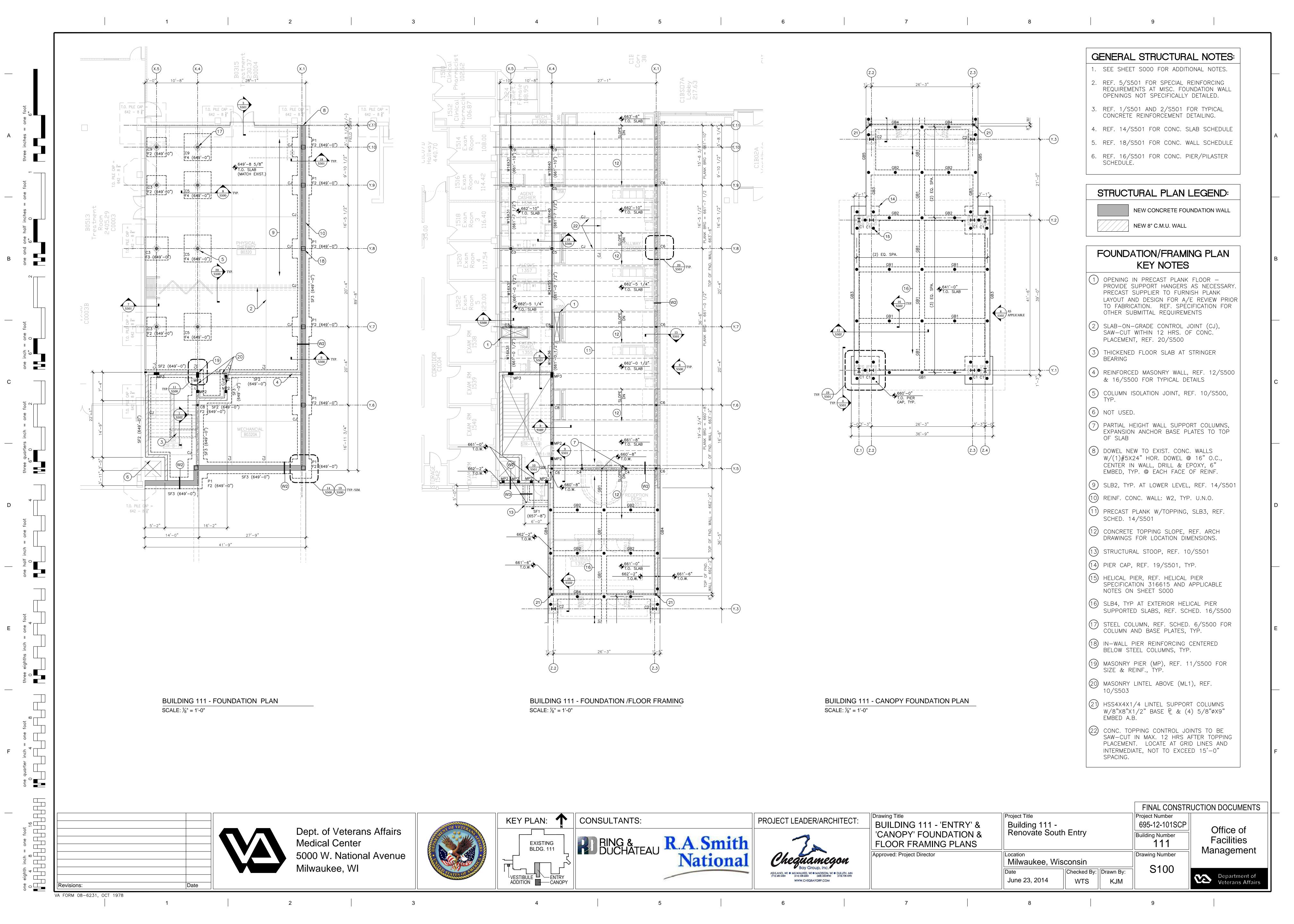
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FINAL CONSTRUCTION DOCUMENTS

Department of Veterans Affairs



BUILDING 70 - CORRIDOR VESTIBULE FOUNDATION

SCALE: ½" = 1'-0"

BUILDING 70 - CORRIDOR VESTIBULE FRAMING PLAN

SCALE: ½" = 1'-0"

GENERAL STRUCTURAL NOTES:

1. SEE SHEET SOOO FOR ADDITIONAL NOTES.

STRUCTURAL PLAN LEGEND:

NEW CONCRETE FOUNDATION WALL

NEW 8" C.M.U. WALL

BUILDING TO VESTIBULE KEY **NOTES**

- 1 STRUCT. STOOP & STAIRS: REF. APPLICABLE ARCHITECTURAL DETAILING FOR CONC. STAIR & RAILING REQUIREMENTS.
- (2) SLB1, TYP. AT INTERIOR VESTIBULE
- 3 REF. ARCH. DRAWINGS FOR LOCATION OF WORK POINTS.
- (4) SAW-CUT & REMOVE T.O.W. TO ELEV. INDICATED, SLAB OVER-POUR MONOLITHIC IN SLB1 REF. ARCH DRAWINGS FOR EXACT LOCATION AND ADDITIONAL DEMO NOTES.
- (5) SKEWED BOLTED/WELDED CONNECTION, REF. 8/S503 FOR BOLT AND PLATE REQUIREMENTS, SIM./TYP.
- 6 METAL DECK TYPE "D1", REF. 6/S503 FOR TYPE AND ATTACHMENT.
- 7 DRILL & EPOXY #3X24" BAR, 5" EMBED INTO EXISTING CMU, GROUT CORE SOLID FULL HEIGHT IF NOT A SOLID GROUTED, VERT. SPACING 16" O.C. MAX. AT COURSING
- (8) REF. ARCH. DRAWINGS FOR LOCATION OF SLAB OVER POUR CONSTRUCTION JOINT -LOCATE BELOW DOOR THRESHOLD.
- 9 EXISTING CONC. WALK TO REMAIN
- (10) C8X11.5 CONT. AROUND PERIM.

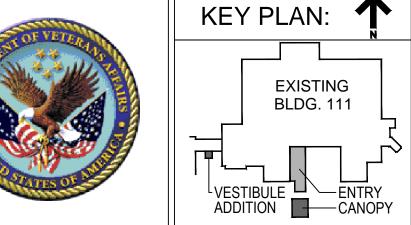
one eighth inch = one foot

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Revisions:

VA FORM 08-6231, OCT 1978

Dept. of Veterans Affairs Medical Center 5000 W. National Avenue Milwaukee, WI



CONSULTANTS:





Drawing Title BUILDING 70 - CONNECTING CORRIDOR VESTIBULE ADDITION FOUNDATION & FLOOR FRAMING PLANS Approved: Project Director

Project Title Building 111 -Renovate South Entry Location
Milwaukee, Wisconsin

June 23, 2014

695-12-101SCP Building Number Drawing Number

Department of Veterans Affairs

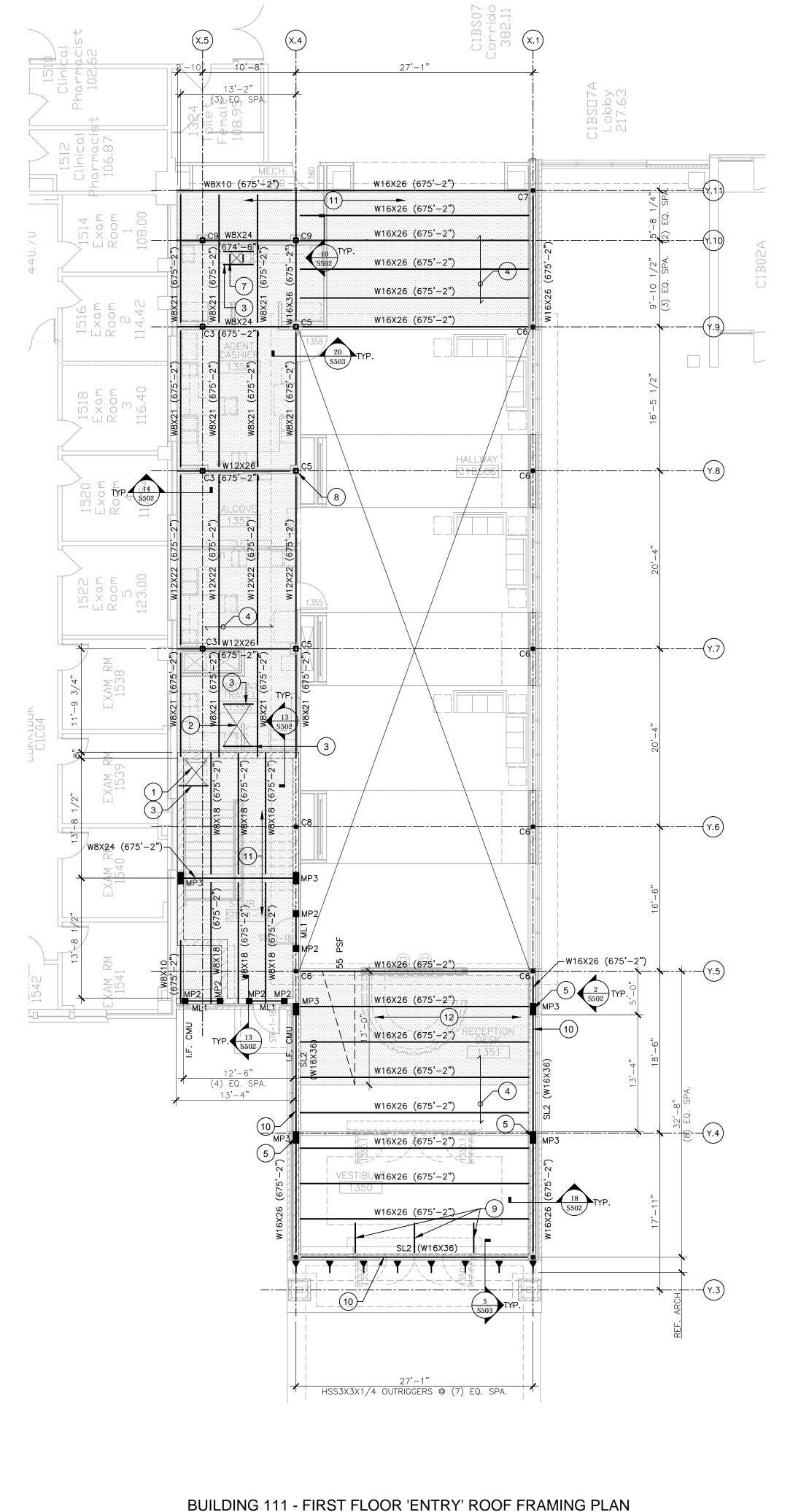
Office of

Facilities

Management

S110 Checked By: Drawn By: KJM

FINAL CONSTRUCTION DOCUMENTS



SCALE: ½" = 1'-0"

W12X14 (672'-2"), TYP. W12X45 4672'-6") W12X22 (672'-2") W12X22 (672'-2") W12X22 (672'-2") W12X22 (672'-2") C12X20.7 (672'-2") 1'-5 3/4" 23'-3 1/2"

GENERAL STRUCTURAL NOTES:

1. SEE SHEET SOOO FOR ADDITIONAL NOTES.

STRUCTURAL PLAN LEGEND:

NEW CONCRETE FOUNDATION WALL

NEW 8" C.M.U. WALL

LOW ROOF FRAMING KEY NOTES

- (1) ROOF ACCESS HATCH
- (2) ROOF OPENING FOR MECHANICAL EQUIP.
- (3) C8X11.5 INTERMEDIATE FRAMING REQUIRED AT ROOF DECK OPENINGS AND AS REQUIRED FOR ACCESS HATCH AND/OR MECHANICAL EQUIPMENT CURB SUPPORT — VERIFY LOCATIONS BASED ON FINAL EQUIPMENT SELECTED PRIOR TO STEEL FABRICATION.
- (4) METAL DECK TYPE "D1", REF. 6/S503 FOR ATTACHMENT.
- (5) SPLICE W16X26 & SL2, MIN STEEL LINTEL BEARING ON CMU=16".
- (6) STEEL MOMENT CONNECTION DESIGNED & DETAILED BY STEEL FABRICATOR, AT EXPENSE TO GC - REF. SPECIFICATION & GENERAL NOTES FOR SUBMITATTAL REQUIREMENTS, TYP.
 - A. TYPICAL E-W DESIGN VALUES (SERVICE LOADS) V=15 KIPS M=61 KIP-FT
- B. TYPICAL N-S DESIGN VALUES (SERVICE V=3 KIPS M=6 KIP-FT
- (7) REF. 12/S503 FOR ROOF OPENING FRAMING AT MISC. OPENINGS FOR EQUIPMENT < 200#
- (8) REF. 9/S503 FOR SHEAR PLATE BOLTED BEAM/COLUMN CONNECTIONS, TYP.
- 9 W12X16 (T.O. BEAMS 675'-0") AT 1/4 POINTS
- (10) STEEL LINTEL (SL2), REF. 10/S503
- 11) UNIFORM SUPERIMPOSED SNOW LOAD = 45 PSF OVER SHADED AREA
- (12) TAPERED SUPERIMPOSED SNOW LOAD SOUTH OF CLEARSTORY AS INDICATED OVER SHADED

BUILDING 111 - FIRST FLOOR 'CANOPY' ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

one eighth inch = one foot

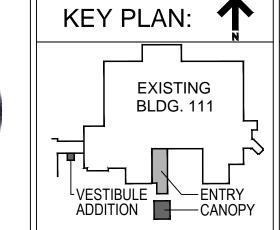
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Revisions:

VA FORM 08-6231, OCT 1978

Dept. of Veterans Affairs Medical Center 5000 W. National Avenue Milwaukee, WI









Drawing Title
BUILDING 111 - 'ENTRY' &
'CANOPY' ROOF
'CANOPY' ROOF FRAMING PLANS
Approved: Project Director

	Project Title
	Building 111 -
	Renovate South Entry
	Transfer Geam Limy
=	
	Location

695-12-101SCP Building Number Drawing Number

S201

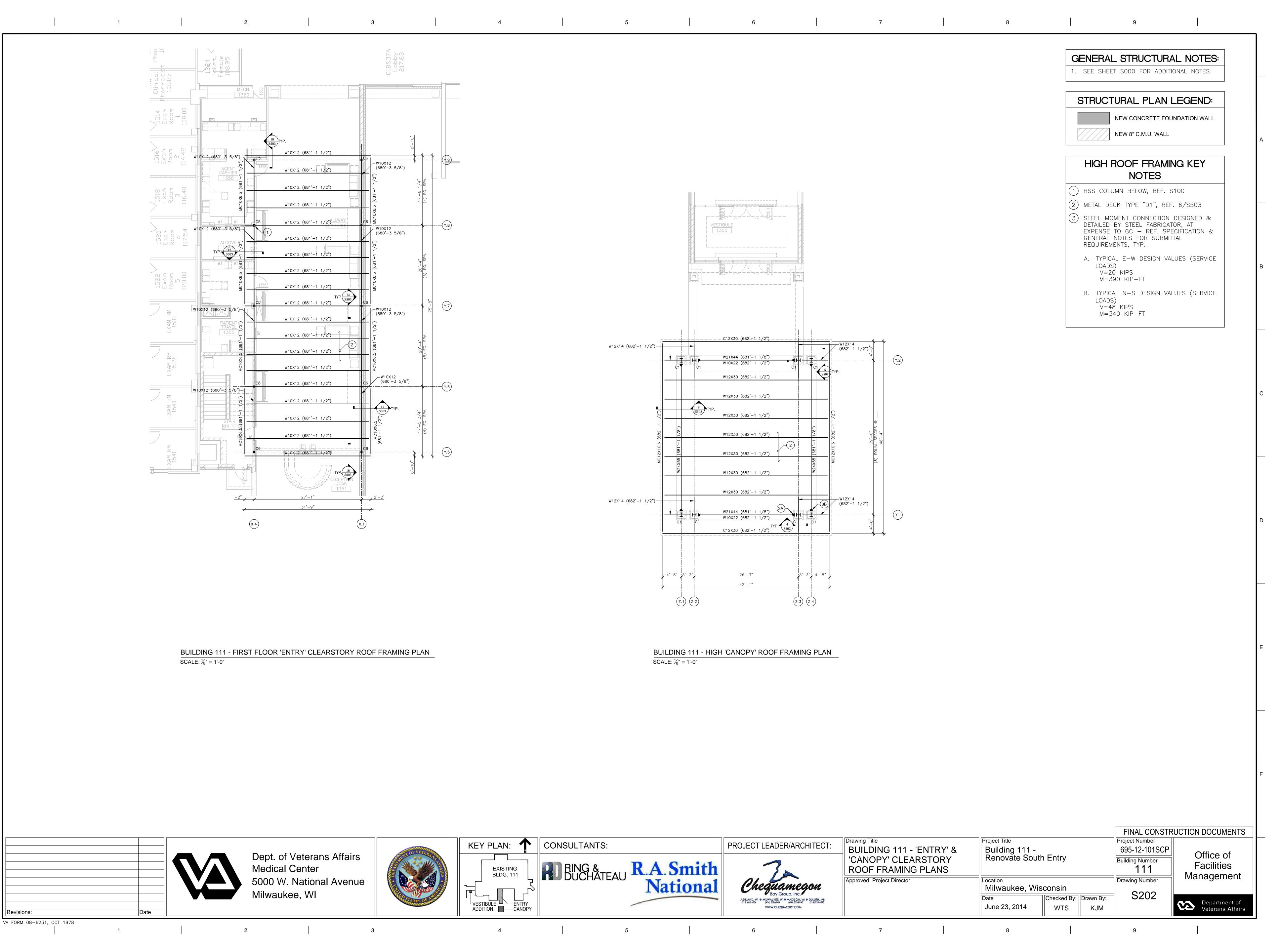
Office of **Facilities** Management

FINAL CONSTRUCTION DOCUMENTS

Department of Veterans Affairs

Chequamegon
Bay Group, Inc.

Milwaukee, Wisconsin Checked By: Drawn By: June 23, 2014 KJM



one eighth inch = one foot

0 4 8 16

